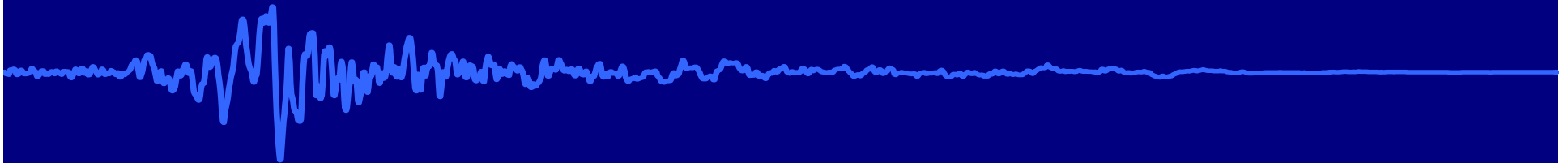


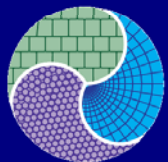
Validity of the pseudostatic surface assumption for evaluating seismically-induced deformation in slopes



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**First International
FLAC/DEM Symposium on Numerical Modeling**



Introduction

“Serviceability” governed by **deformation (δ) not FS!**

Deformation-Based Procedures:

“Rigid-Block” Method
Newmark (1965)

“Decoupled” Method
Makdisi & Seed (1978)

Assumes deformations occur on the *critical “pseudostatic” surface* (by limit-equilibrium)

Questionable assumption since...



Objective!

Quantify how dynamic response affects the failure mechanism geometry

Ground Motion Database

124 earthquake recordings from 24 events

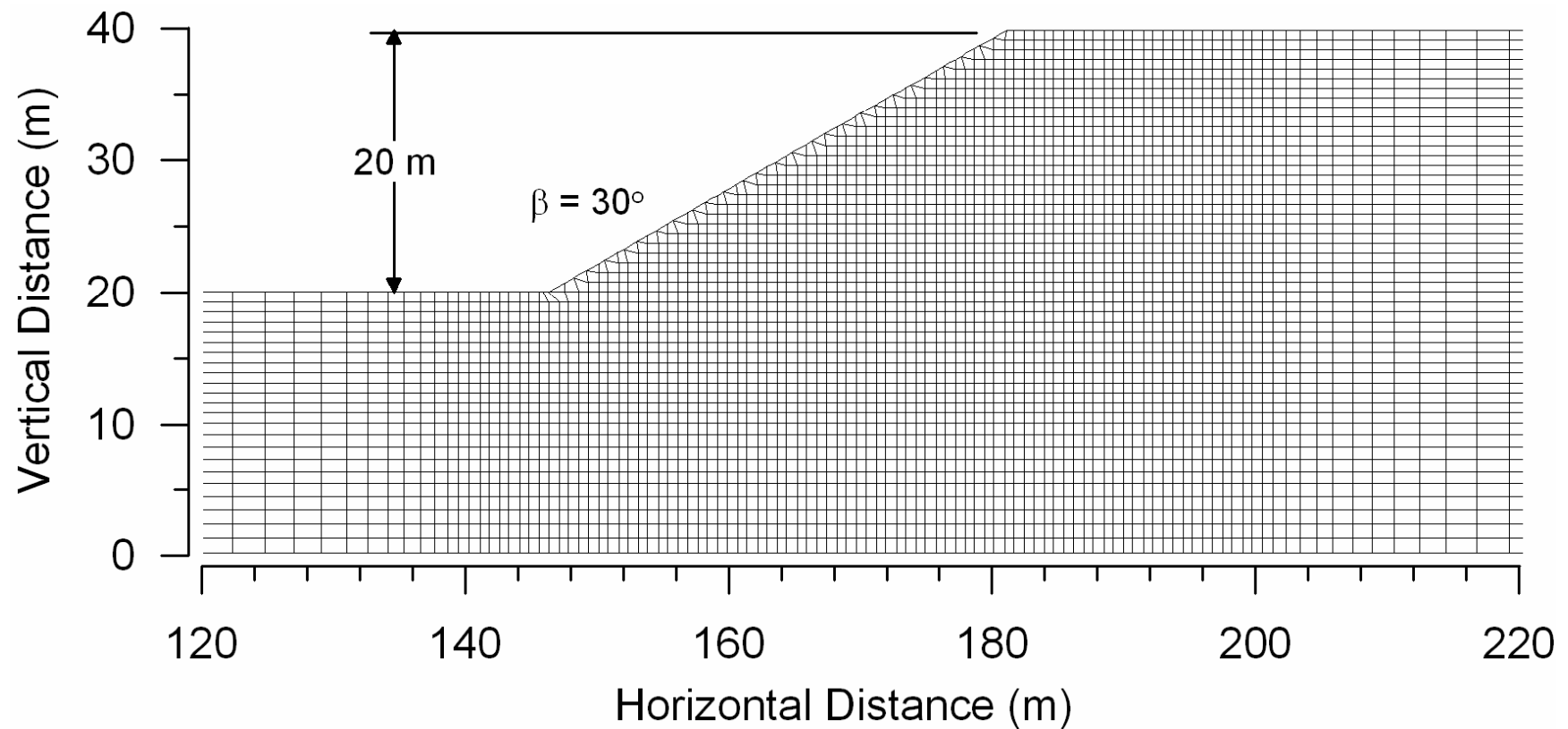
- Mean-square frequency (f_m) ranges from 0.85 to 8.1 Hz

7 single-frequency sine motions ($f = 1.25$ to 5 Hz)

Motions scaled to uniform peak acceleration

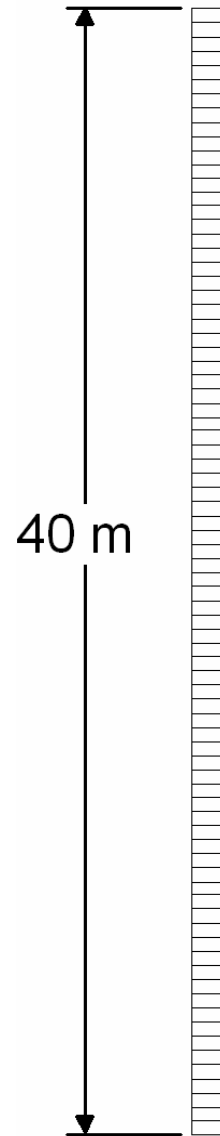
FLAC Models

Simple, homogeneous slope

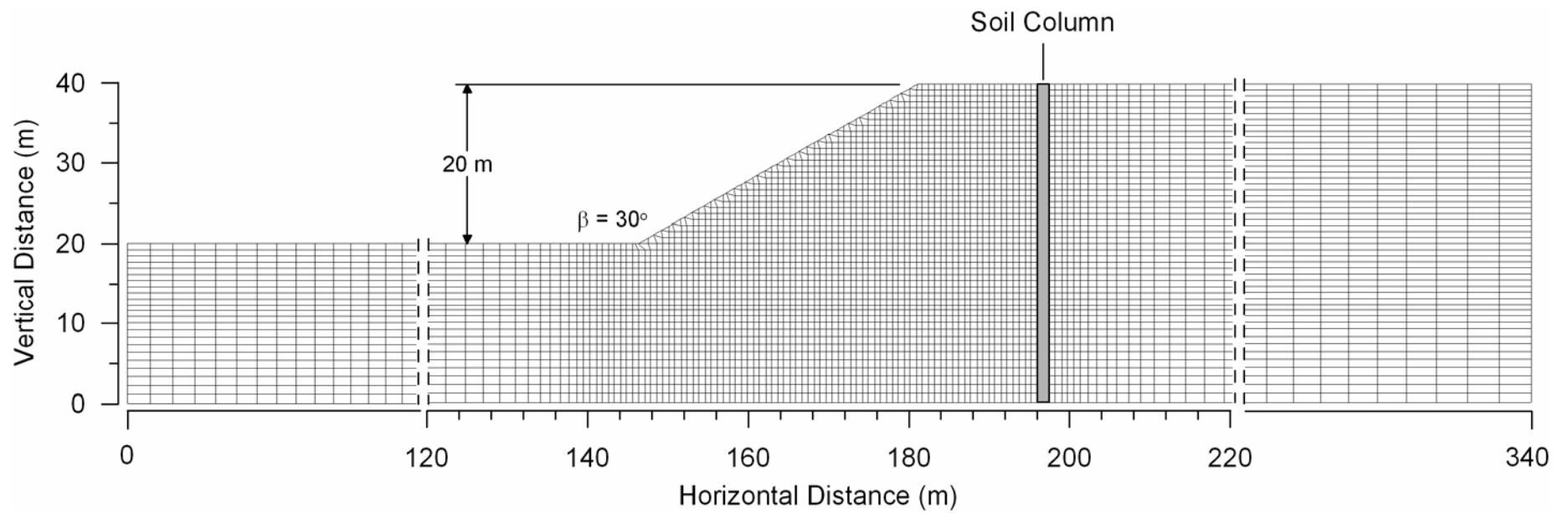


FLAC Models

Soil Column



FLAC Models



FLAC Analyses

Simple, homogeneous slope

- “Pseudostatic-stepping” method

Compare FLAC to the LEM “pseudostatic” surface

- Fully-coupled non-linear dynamic analysis

(Rayleigh, $\xi = 2.5\%$)

Compare the fully-coupled surfaces to the FLAC “pseudostatic” surface (“pseudostatic-stepping”)

Soil column

- Linear, elastic dynamic analysis (Rayleigh, $\xi = 2.5\%$)

Determine how dynamic response affects the depth of the “critical surface”

Simple, Homogeneous Slope

Frictional-cohesive material

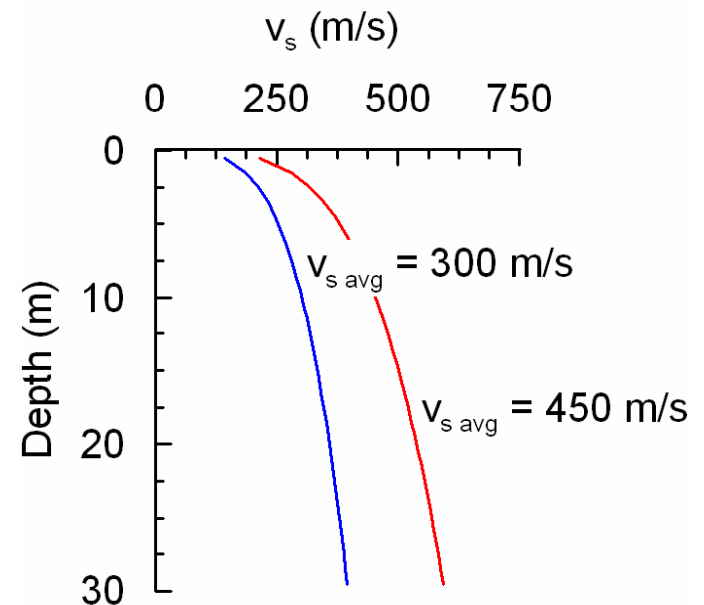
- Strain-softening model
- $\phi = 30^\circ$, $c \approx 210$ psf with $SI = 1.05$
- Non-associated flow rule ($\psi = 0^\circ$)

Purely-cohesive material

- Mohr-Coulomb model
- $\phi = 0^\circ$, $s_u/p = 0.33$

2 shear wave velocity profiles

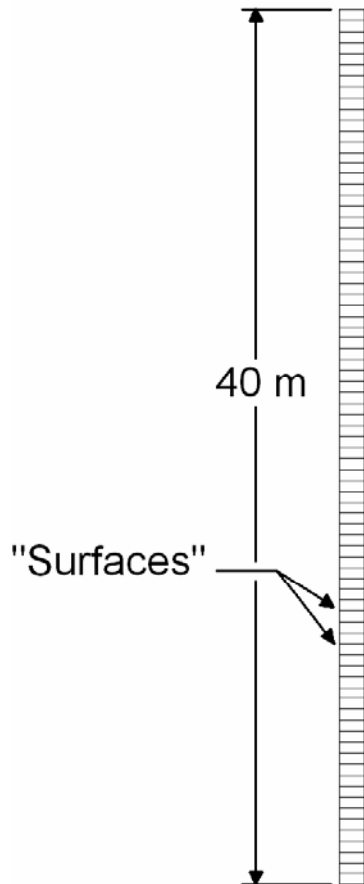
- Increasing with depth
- $v_s = 300$ and 450 m/s



$$G_{\max} = 21.7 p_a K \left(\frac{\sigma'}{p_a} \right)^{0.5}$$

Seed & Idriss (1970)

Soil Column



Linear-elastic model

- 4 shear wave velocity profiles
- $v_s = 150, 300, 450$ and 600 m/s
- Seed & Idriss (1970)

$$\text{HEA}(t) = \frac{\tau(t)}{\sigma_v} g$$

$$k_{\max} = \max[\text{HEA}(t)]$$

“Seismic Demand”

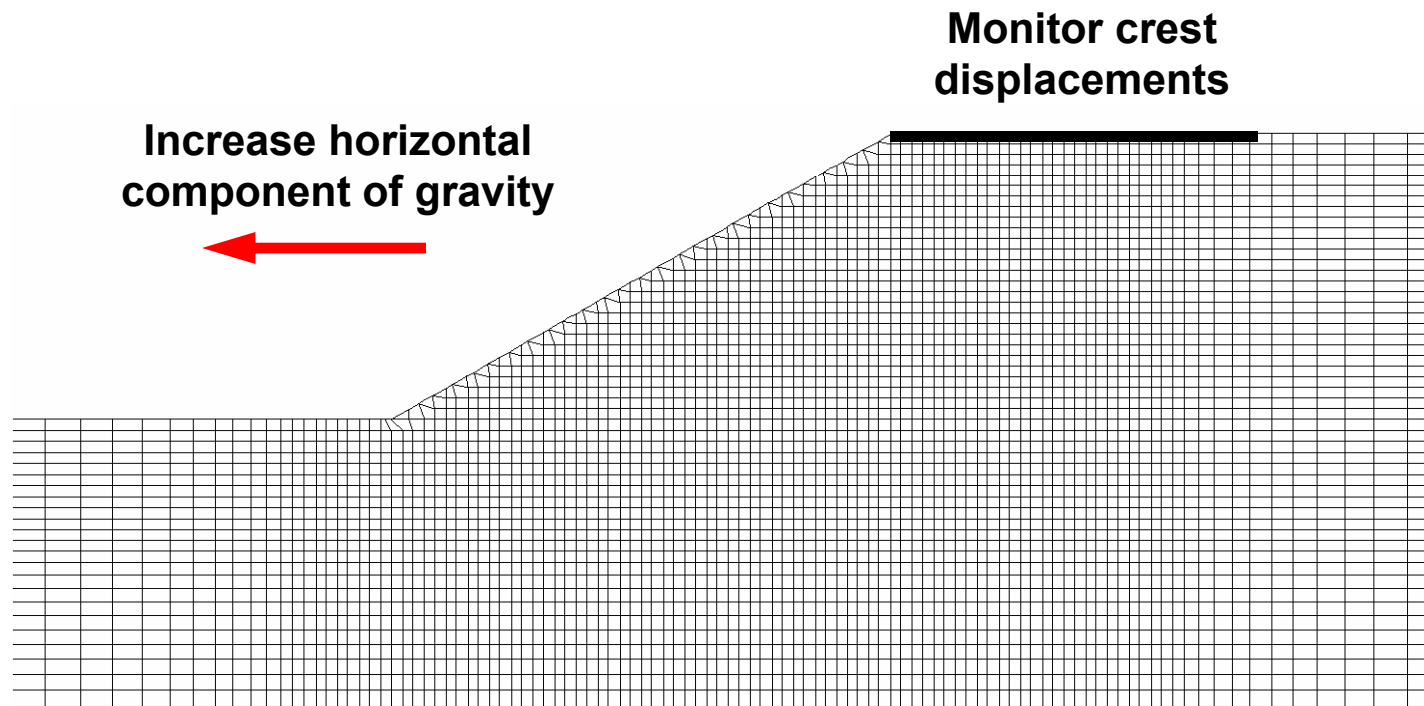
$$k_y = \frac{\tau_{\max}(z)}{\sigma_v} g$$

“Seismic Resistance”

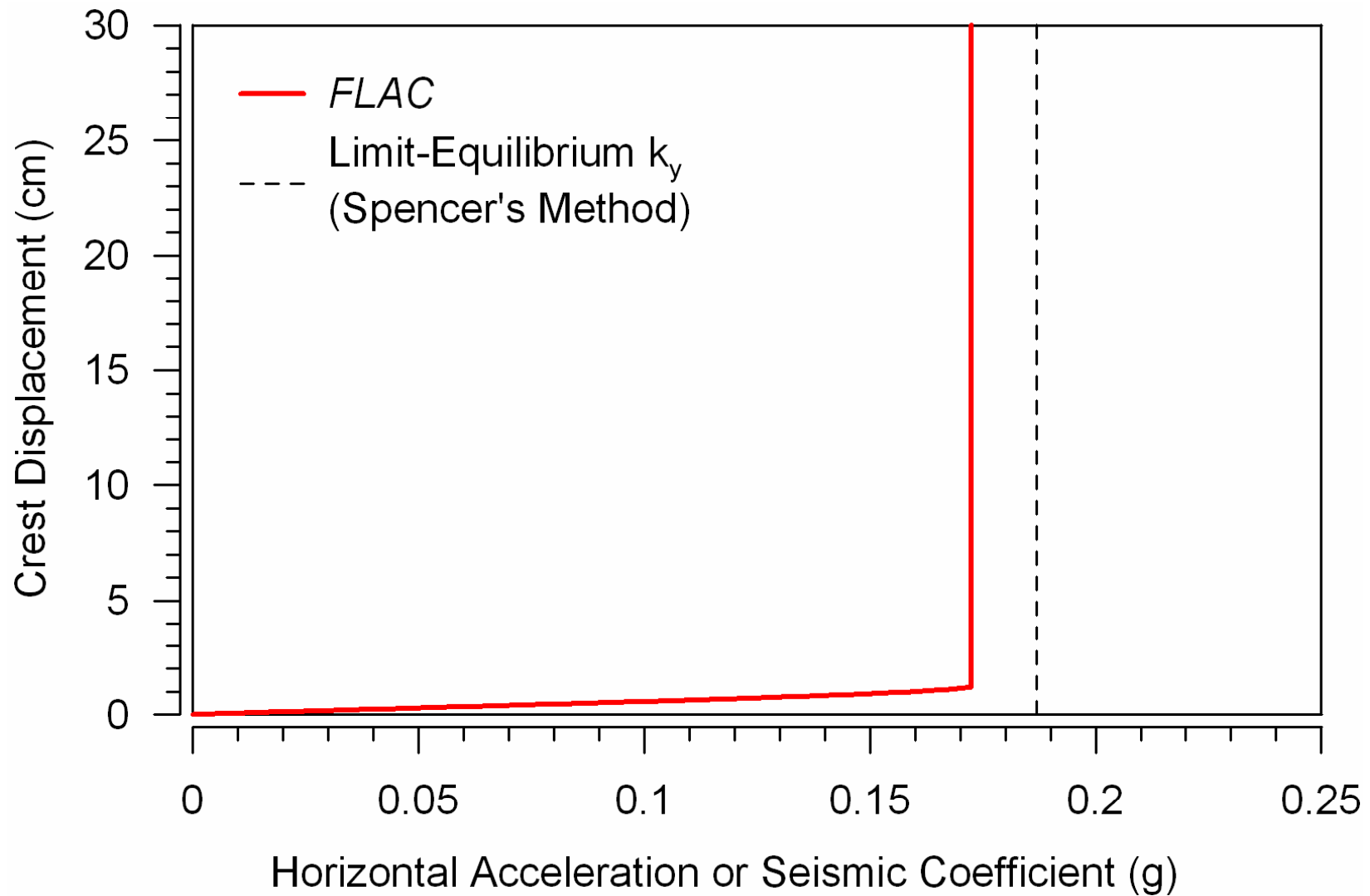
Stability ratio: k_y/k_{\max}

Depth at *minimum* k_y/k_{\max} is
most unstable (critical)!

“Pseudostatic-Stepping” Analysis

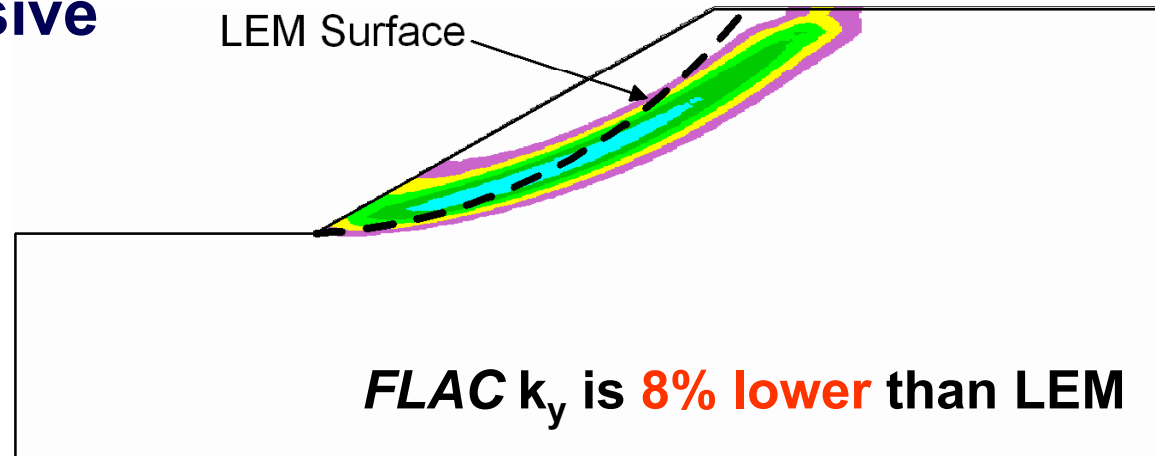


“Pseudostatic-Stepping” Analysis

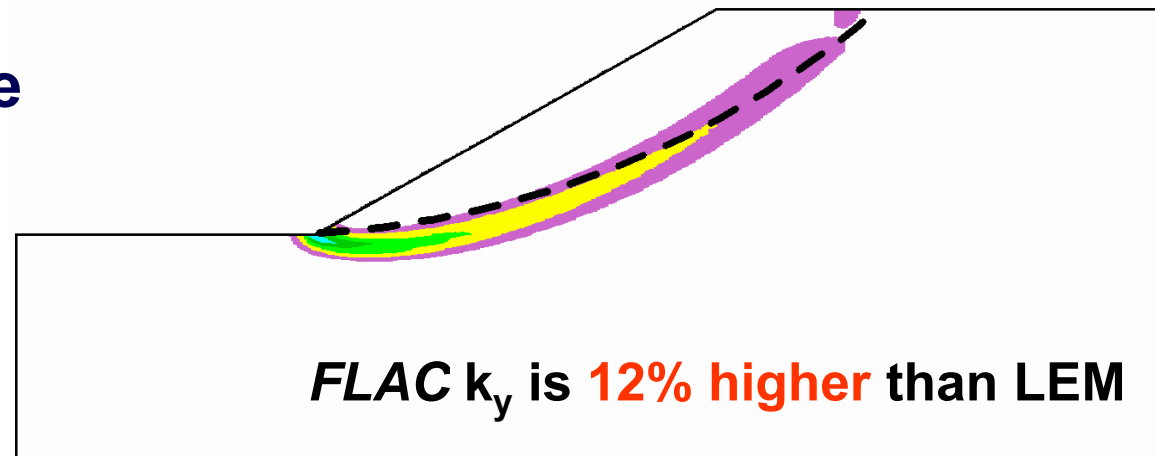


“Pseudostatic-Stepping” Analysis

Frictional-cohesive



Purely-cohesive



Quantifying the Results

Dynamic response conditions:

Wavelength ratio $\frac{\lambda}{H} = \frac{V_{s \text{ avg}}}{f_m H}$

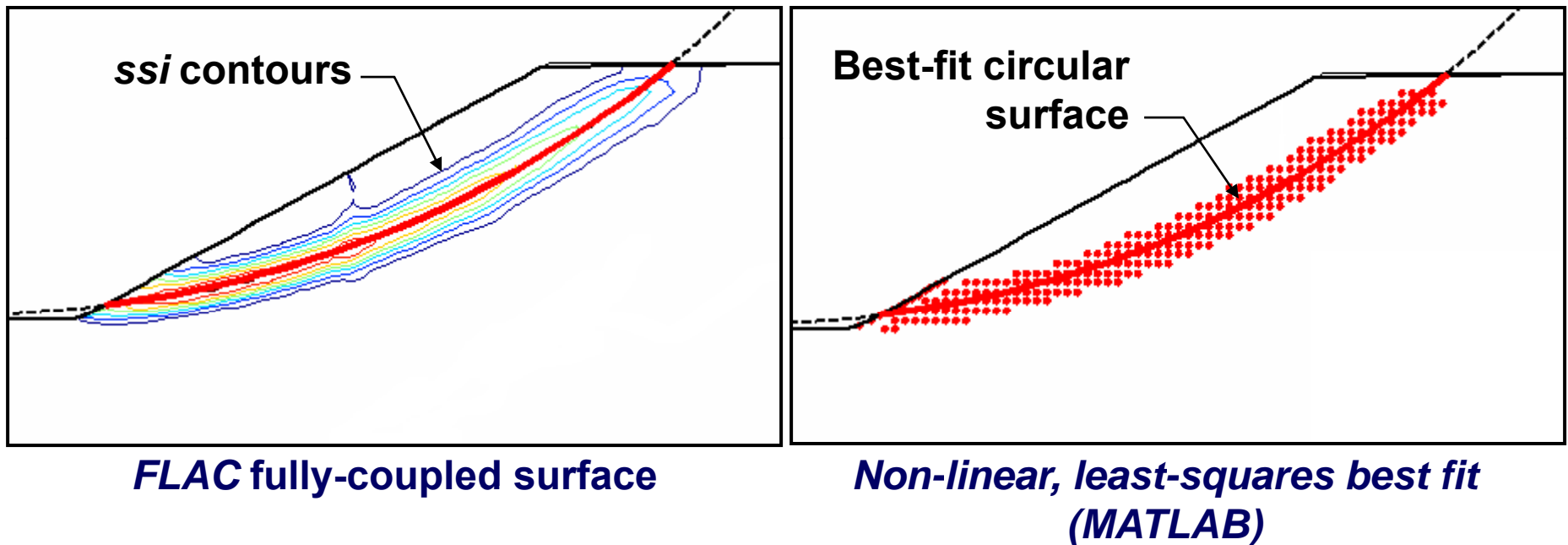
Simple relationship...

Low λ/H means High freq. motions

High λ/H means Low freq. motions

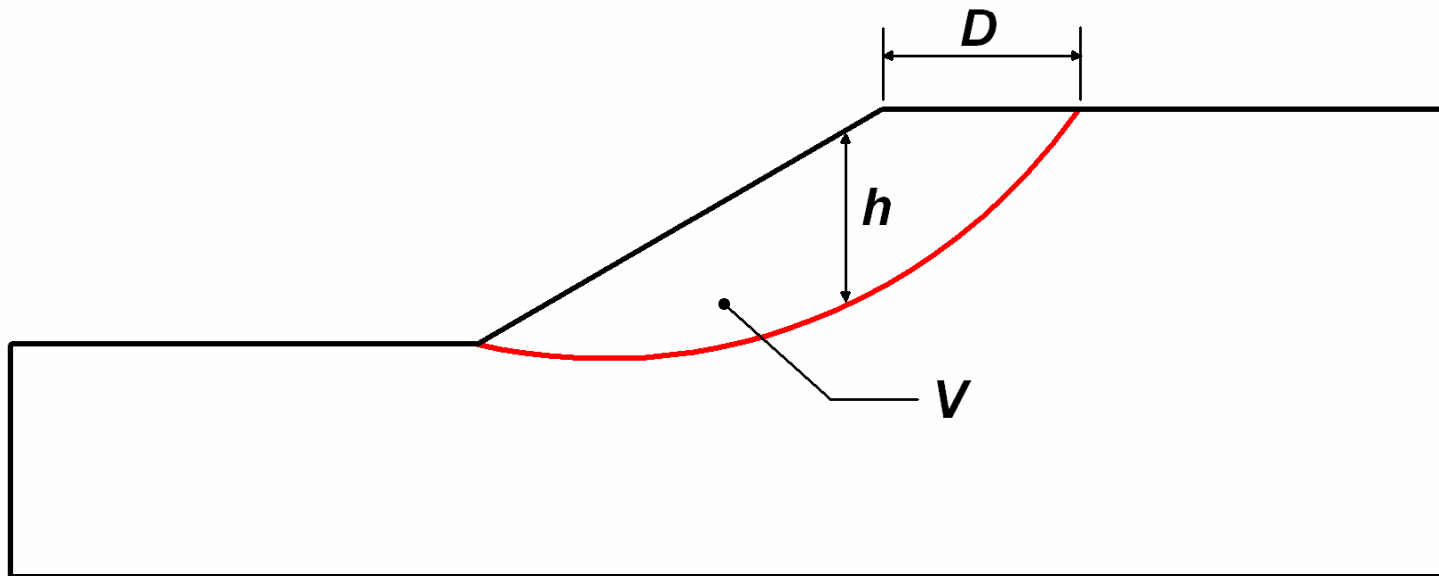
Quantifying the Results

Failure mechanism geometry:



Quantifying the Results

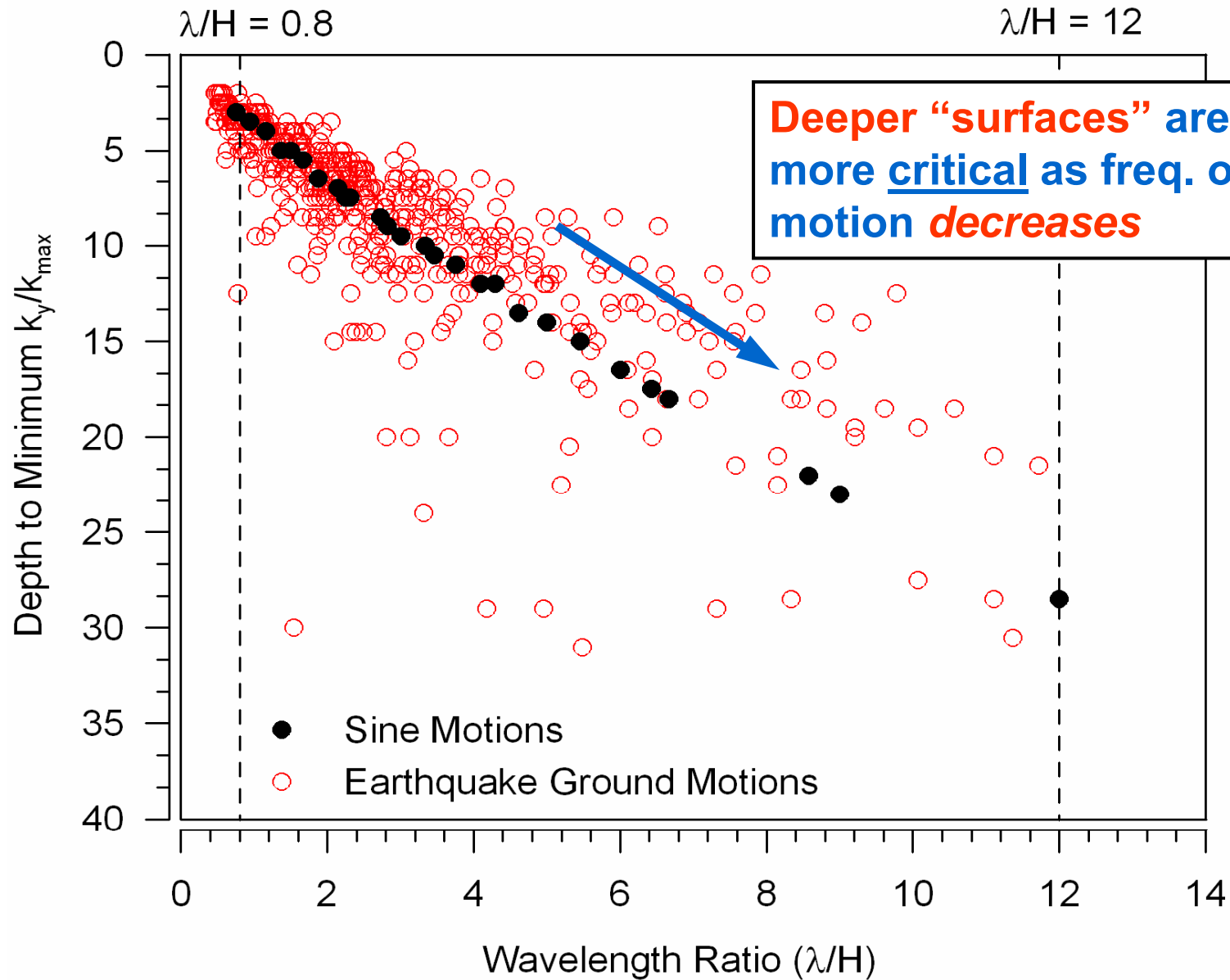
Failure mechanism geometry:



Surface descriptors:

- Maximum slide mass thickness (h)
- Crest distance (D)
- Volume (V) (per unit length)

Soil Column Results



Simple Slope Results

$$\text{Ratio} = \frac{\text{"coupled"}}{\text{"pseudostatic"}}$$

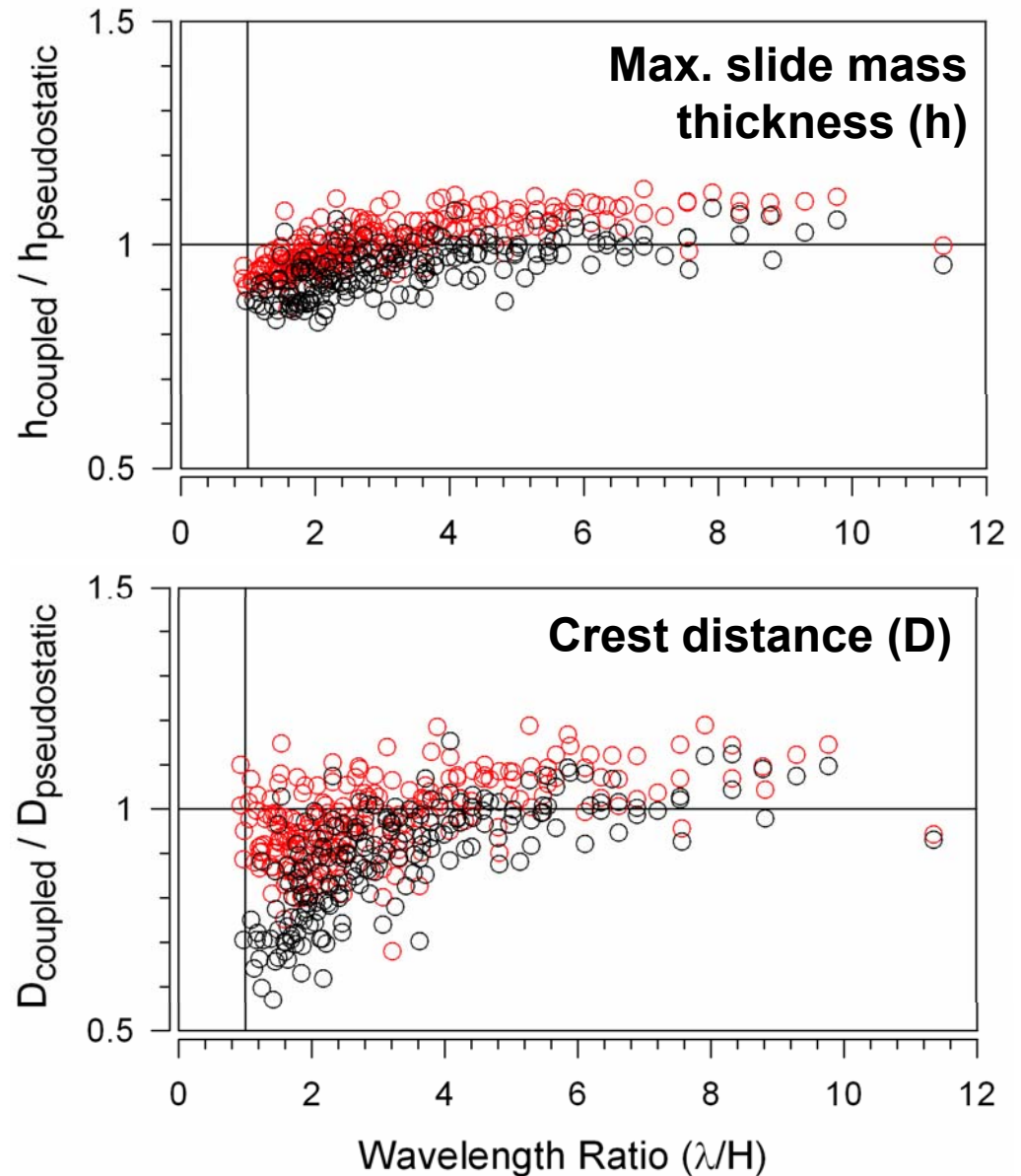
Similar trend to soil column results

For $1 < \frac{\lambda}{H} < 4$

Coupled surface "shallower" than pseudostatic surface

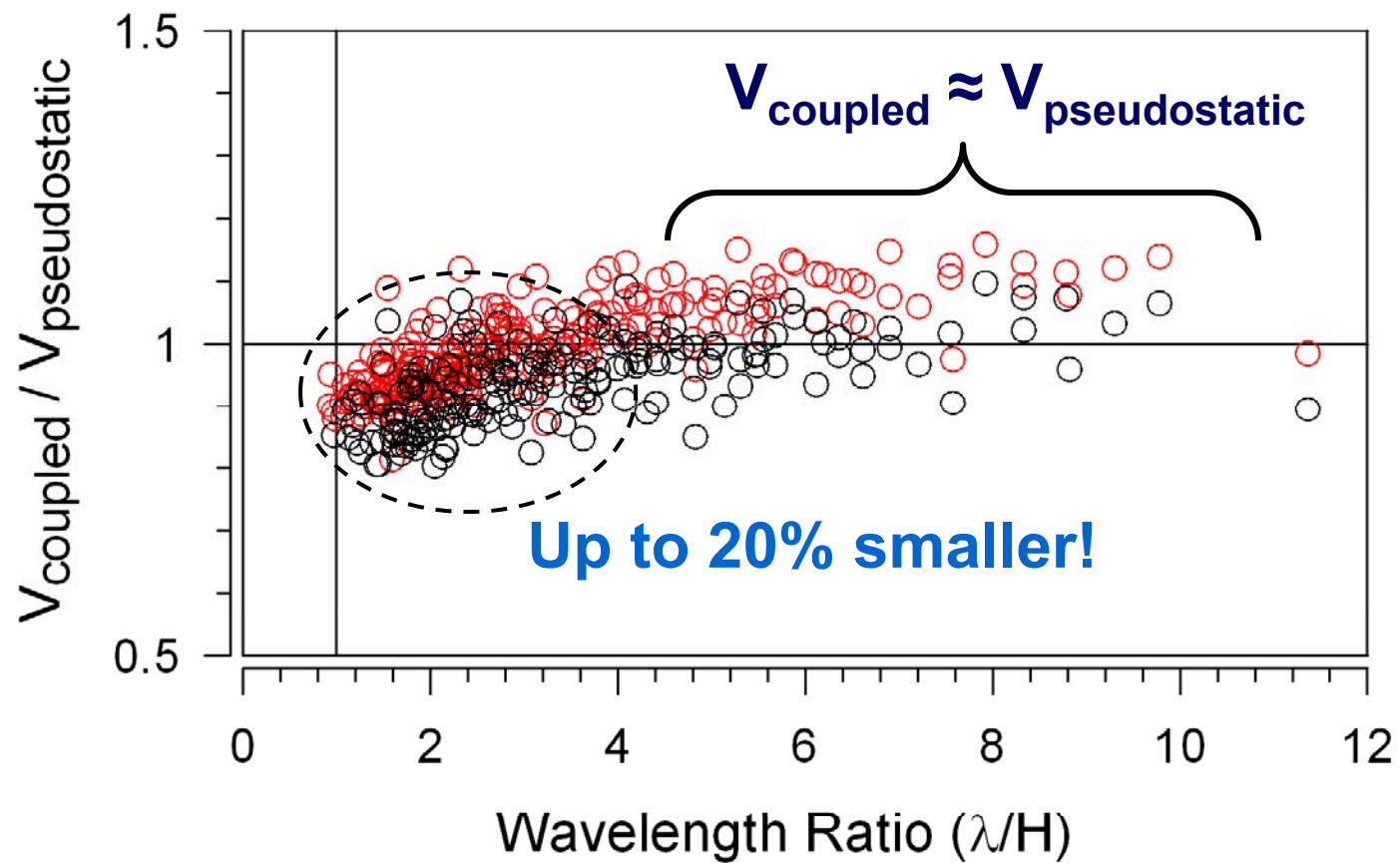
For $\frac{\lambda}{H} > 4$

Coupled surface "matches" the pseudostatic surface

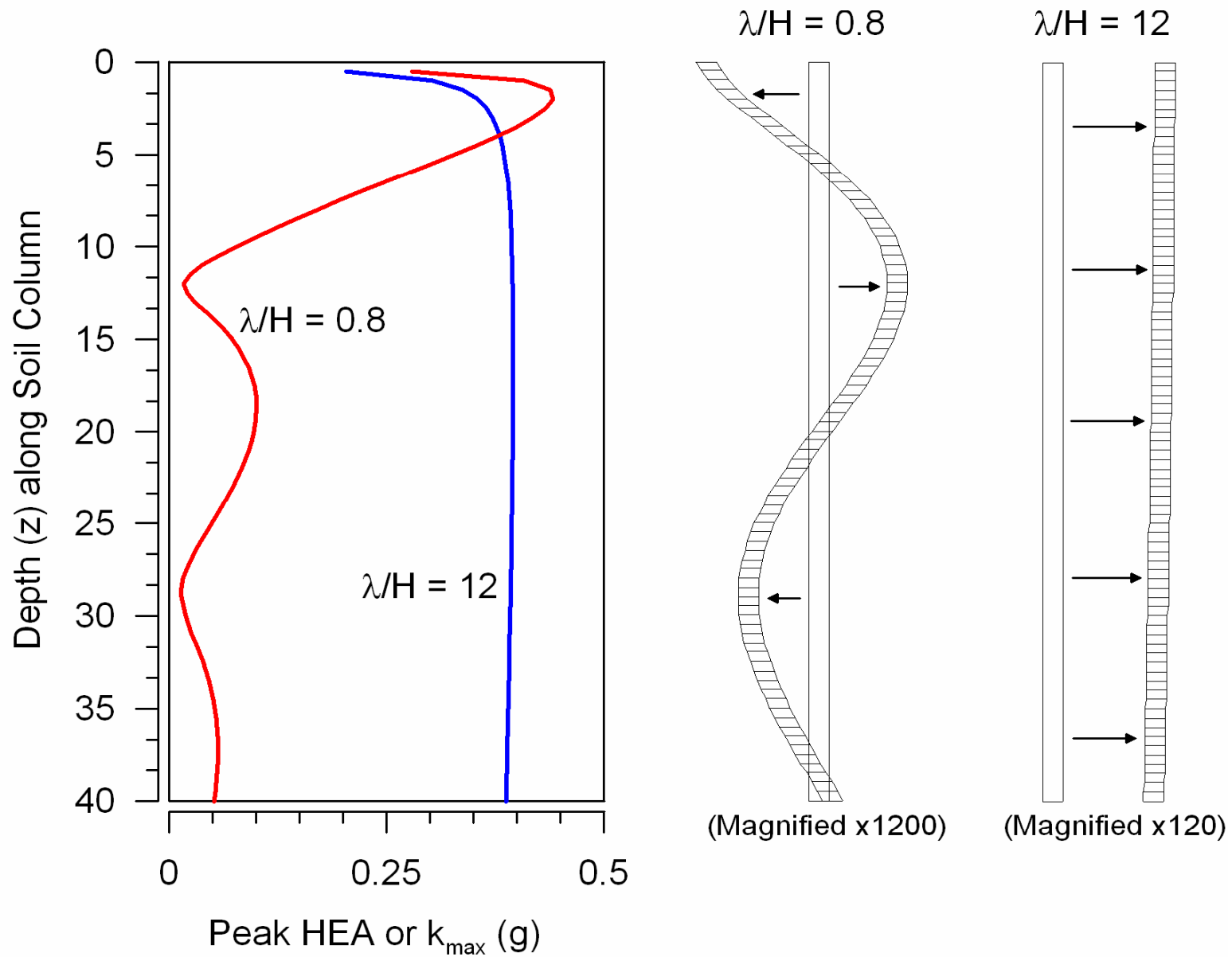


Simple Slope Results

Slide mass volume (V)



Discussion

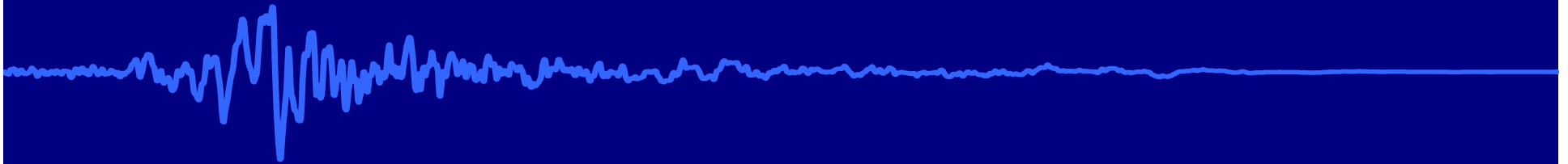


Conclusions

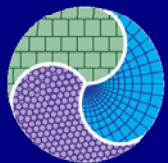
- Developed the “pseudostatic-stepping” method to find pseudostatic surface and k_y – **no a priori assumptions as with LEM.**
- Dynamic response has a *more pronounced* effect at λ/H ratios between 1 and 4
 - Surfaces are up to **20% shallower** and **smaller** in volume
 - Fully-coupled analyses are recommended
- Dynamic response has *less of an effect* at λ/H greater than 4
 - Surfaces “approximate” the **FLAC pseudostatic surface**
 - Other deformation-based procedures (rigid-block or decoupled) can be used - “**pseudostatic surface assumption**” applies



Thank you!



Questions?



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